UNIVERSITY OF MARYLAND Department of Civil and Environmental Engineering College Park Campus Fall Semester 2002 Solution to QUIZ #1



For the reinforced concrete beam cross section shown in the figure, determine

- 1- The amount of steel required to create the balanced condition.
- 2- Whether this beam is balanced, underreinforced, or overreinforced? Why?
- 3- The practical moment capacity ϕM_n of the beam according to the ACI Code.

Use $f_y = 40,000$ psi for steel and $f'_c = 3,000$ psi for concrete.

**** SOLUTION ****

1- <u>Amount of Steel:</u>

From Table 2, $\rho_{\text{max}} = 0.0278$, therefore, $\rho_b = \frac{\rho_{\text{max}}}{0.75} = \frac{0.0278}{0.75} = 0.03707$

Hence, Steel required for balanced condition = $A_{sb} = \rho_b bd = 0.03707(11)(20) = 8.16 \text{ in}^2$

(Note: ρ_b can also be obtained using $\rho_b = \frac{0.85 f_c' \beta_1}{f_y} \left(\frac{87,000}{f_y + 87,000} \right)$

2- <u>The Condition of the Beam:</u>

From Table 1, the area for one #10 bar = 1.27 in² Therefore, $A_s = 6 \times 1.27 = 7.62$ in²

The beam is underreinforced because

$$(A_s = 7.62 \text{ in}^2) < (A_{sb} = 8.16 \text{ in}^2)$$

3- <u>Practical moment capacity of the beam:</u>

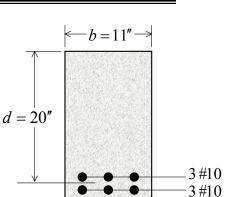
According to the ACI Code, the practical moment capacity ϕM_n of the beam should be determined based on $A_{s,max}$, and not $A_s = 7.62 \text{ in}^2$. Therefore,

$$A_{s,\max} = \rho_{\max} bd = 0.0278(11)(20) = 6.12 \text{ in}^2$$

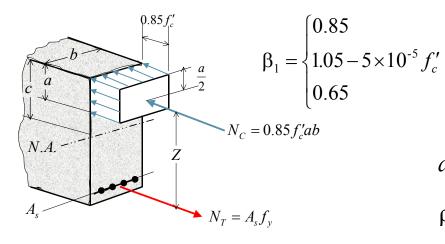
$$a = \frac{A_{s,\max} f_y}{0.85 f'_c b} = \frac{6.12(40)}{0.85(3)(11)} = 8.73 \text{ in.}$$

$$Z = 20 - \frac{a}{2} = 20 - \frac{8.73}{2} = 15.64 \text{ in.}$$

$$\varphi M_n = (0.90)(A_{s,\max} f_y Z) = 0.9(6.12)(40)(15.64) = 3,445.8 \text{ in - kips} = 287 \text{ ft - kips}$$



Formulas, Tables, and Figures



for
$$f'_{c} \le 4,000 \text{ psi}$$

for 4,000 psi < $f'_{c} \le 8,000 \text{ psi}$
for $f'_{c} > 8,000 \text{ psi}$

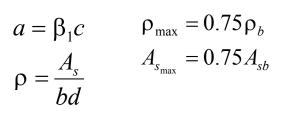


Table 1.	ASTM Standard	- English	Reinforcing Bars

		-	*
Bar Designation	Diameter in	Area in ²	Weight Ib/ft
#3 [#10]	0.375	0.11	0.376
#4 [#13]	0.500	0.20	0.668
#5 [#16]	0.625	0.31	1.043
#6 [#19]	0.750	0.44	1.502
#7 [#22]	0.875	0.60	2.044
#8 [#25]	1.000	0.79	2.670
#9 [#29]	1.128	1.00	3.400
#10 [#32]	1.270	1.27	4.303
#11 [#36]	1.410	1.56	5.313
#14 [#43]	1.693	2.25	7.650
#18 [#57]	2.257	4.00	13.60

$$\rho_b = \frac{0.85 f_c' \beta_1}{f_y} \left(\frac{87,000}{f_y + 87,000} \right)$$

Note: Metric designations are in brackets

Table 2. Design Constants

	-					
	$\begin{bmatrix} 3 \sqrt{f'} & 200 \end{bmatrix}$	Recommended Design Values				
$f_{\rm c}'({\rm psi})$	$\left\lfloor \frac{\frac{s\sqrt{y_c}}{f_y}}{f_y} \ge \frac{200}{f_y} \right\rfloor$	$\rho_{max} = 0.75 \ \rho_b$	ρ	\overline{k} (ksi)		
$F_y = 40,000 \text{ psi}$						
3,000	0.0050	0.0278	0.0135	0.4828		
4,000	0.0050	0.0372	0.0180	0.6438		
5,000	0.0053	0.0436	0.0225	0.8047		
6,000	0.0058	0.0490	0.0270	0.9657		
$F_y = 50,000 \text{ psi}$						
3,000	0.0040	0.0206	0.0108	0.4828		
4,000	0.0040	0.0275	0.0144	0.6438		
5,000	0.0042	0.0324	0.0180	0.8047		
6,000	0.0046	0.0364	0.0216	0.9657		
$F_y = 60,000 \text{ psi}$						
3,000	0.0033	0.0161	0.0090	0.4828		
4,000	0.0033	0.0214	0.0120	0.6438		
5,000	0.0035	0.0252	0.0150	0.8047		
6,000	0.0039	0.0283	0.0180	0.9657		
$F_y = 75,000 \text{ psi}$						
3,000	0.0027	0.0116	0.0072	0.4828		
4,000	0.0027	0.0155	0.0096	0.6438		
5,000	0.0028	0.0182	0.0120	0.8047		
6,000	0.0031	0.0206	0.0144	0.9657		
	$\begin{array}{r} 4,000\\ 5,000\\ 6,000\\ \hline \\ 3,000\\ 4,000\\ 5,000\\ \hline \\ 3,000\\ 4,000\\ 5,000\\ \hline \\ 3,000\\ 6,000\\ \hline \\ 3,000\\ 4,000\\ 5,000\\ \hline \end{array}$	$\begin{tabular}{ c c c c c } \hline f_y & f_y \\ \hline \hline \\ \hline $	$ \begin{array}{c c c c c c c c c } \hline F_y & f_y & f_y & F_y = 40,000 \ \text{psi} \\ \hline F_y = 40,000 \ \text{psi} \\ \hline F_y = 40,000 \ 0.0050 & 0.0278 \\ \hline 4,000 & 0.0050 & 0.0372 \\ \hline 5,000 & 0.0053 & 0.0436 \\ \hline 6,000 & 0.0058 & 0.0490 \\ \hline F_y = 50,000 \ \text{psi} \\ \hline 3,000 & 0.0040 & 0.0206 \\ \hline 4,000 & 0.0040 & 0.0275 \\ \hline 5,000 & 0.0042 & 0.0324 \\ \hline 6,000 & 0.0046 & 0.0364 \\ \hline F_y = 60,000 \ \text{psi} \\ \hline 3,000 & 0.0033 & 0.0161 \\ \hline 4,000 & 0.0035 & 0.0252 \\ \hline 6,000 & 0.0035 & 0.0252 \\ \hline 6,000 & 0.0037 & 0.0116 \\ \hline 4,000 & 0.0027 & 0.0116 \\ \hline 4,000 & 0.0027 & 0.0155 \\ \hline 5,000 & 0.0028 & 0.0182 \\ \hline \end{array} $	$ \begin{array}{c c} f_c'(\mathrm{psi}) & \left[\begin{array}{c} \frac{3\sqrt{y}}{f_y} \geq \frac{200}{f_y} \right] \\ \hline \rho_{\mathrm{max}} = 0.75 \ \rho_{\mathrm{b}} \\ \hline \rho \\ \hline F_y = \mathbf{40,000 \ psi} \\ \hline 3,000 & 0.0050 & 0.0278 & 0.0135 \\ 4,000 & 0.0050 & 0.0372 & 0.0180 \\ 5,000 & 0.0053 & 0.0436 & 0.0225 \\ 6,000 & 0.0058 & 0.0490 & 0.0270 \\ \hline F_y = \mathbf{50,000 \ psi} \\ \hline 3,000 & 0.0040 & 0.0206 & 0.0108 \\ 4,000 & 0.0040 & 0.0275 & 0.0144 \\ 5,000 & 0.0042 & 0.0324 & 0.0180 \\ 6,000 & 0.0046 & 0.0364 & 0.0216 \\ \hline F_y = \mathbf{60,000 \ psi} \\ \hline 3,000 & 0.0033 & 0.0161 & 0.0090 \\ 4,000 & 0.0035 & 0.0252 & 0.0150 \\ 6,000 & 0.0035 & 0.0252 & 0.0150 \\ \hline 6,000 & 0.0037 & 0.0116 & 0.0072 \\ \hline 3,000 & 0.0027 & 0.0116 & 0.0072 \\ \hline 4,000 & 0.0027 & 0.0155 & 0.0096 \\ \hline 5,000 & 0.0028 & 0.0182 & 0.0120 \\ \hline \end{array} $		

Table 3. Strength Reduction Factors

Type of Loading	¢
Bending	0.90
Shear and Torsion	0.85
Compression members (spirally reinforced)	0.75
Compression Members (tied)	0.70
Bearing on Concrete	0.70